WELL MADE

SYSTEM SCAFFOLDING

Since 2001s,

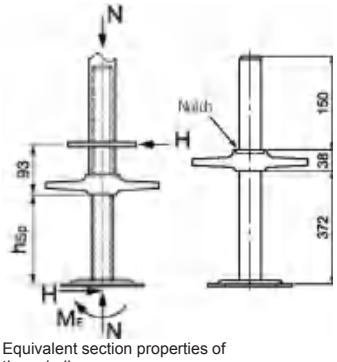
20 Years Scaffolding Manufacture Experience

50,000m2 Automatic ScaffoldingProduction Lines

Own Scaffolding Test House

Wellmade Scaffold， China

BASE PLATE WRBP060



Equivalent section properties of the spindle:
A = 3.84 cm²
W_{el} = 2.61 cm³
W_{pl} = 3.26 cm³
I = 3.74 cm⁴
Material: EN 10219-S235JRH
→ Threaded spindle: $f_{y,k}$ = 280.0 N/mm²

Base plate type	N_{Rd} [kN]	M_{Rd} [kNcm]	V_{Rd} [kN]
Standard	97.7	83.0	36.0
Reinforced	119.9	94.5	44.1
Solid	288.0	157.0	106.0

Table 1. Base plate loading

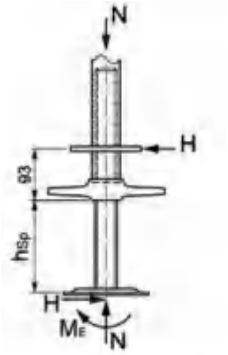
Spindle extension length h_{Sp} [cm]	Permissible vertical spindle load N [kN]* with simultaneous effect of a horizontal load H [kN]																								Perm. horizontal load H [kN] when N = 0
	H = 0.0		H = 0.5		H = 1.0		H = 1.5		H = 2.0		H = 2.5		H = 3.0		H = 3.5		H = 4.0		H = 4.5		H = 5.0		H = 5.5		H = 6.0		
	N_1	N_2	N_1	N_2	N_1	N_2	N_1	N_2	N_1	N_2	N_1	N_2	N_1	N_2	N_1	N_2	N_1	N_2	N_1	N_2	N_1	N_2	N_1	N_2	N_1	N_2	
0	39	53	39	51	39	51	39	51	39	50	39	49	39	49	38	38	38	38	37	37	36	36	36	36	35	35	26.3
5	39	52	39	51	39	50	39	48	38	38	37	37	36	36	35	35	34	34	33	33	32	32	31	31	30	30	7.8
10	39	51	39	49	38	38	37	37	36	36	34	34	33	33	30	30	29	29	28	28	26	26	25	25	-	-	4.6
15	39	49	38	38	36	36	35	35	33	33	31	31	29	29	-	-	-	-	-	-	-	-	-	-	-	-	3.2
20	38	38	36	36	34	34	32	32	29	29	27	27	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2.5
25	37	37	34	34	31	31	28	28	23	23	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2.0
30	35	35	31	31	27	27	21	21	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.7
35	32	32	27	27	21	21	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.5
37	30	30	25	25	17	17	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.4

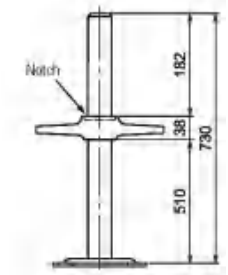
* The permissible vertical loads are calculated by application of the calculation model according to DIN EN 12811-1, para. 10.2.3.2. To obtain the bending stiffness, internal forces and maximum load-bearing capacity,
the birdcage scaffolding with modular dimensions 2.57 x 2.57 m and different lift heights were considered:
2.00 m level height for compression forces in the standard N1 ≤ 39 kN,
1.50 m level height for compression forces in the standard 39 kN < N2 ≤ 54 kN.

(-) With this combination of spindle extension length and horizontal load, the bending stress capacity of the spindle is exceeded.

38—the listed value or better.

BASE PLATE WRBPS060





Equivalent section properties of the spindle:
A = 4.71 cm²
W_{el} = 2.97 cm³
W_{pl} = 3.71 cm³
I = 4.29 cm⁴
Material: EN 10219-S235JRH
→ Threaded spindle: $f_{y,k}$ = 280.0 N/mm²

Table 2. Base plate loading

Spindle extension length h_{Sp} [cm]	Permissible vertical spindle load N [kN]* with simultaneous effect of a horizontal load H [kN]																																		Perm. horizontal load H [kN] when N = 0
	H = 0			H = 0.5			H = 1.0			H = 1.5			H = 2.0			H = 2.5			H = 3.0			H = 3.5			H = 4.0			H = 4.5			H = 5.0			H = 5.5			H = 6.0			
	N_1	N_2	N_3	N_1	N_2	N_3	N_1	N_2	N_3	N_1	N_2	N_3	N_1	N_2	N_3	N_1	N_2	N_3	N_1	N_2	N_3	N_1	N_2	N_3	N_1	N_2	N_3	N_1	N_2	N_3	N_1	N_2	N_3	N_1	N_2	N_3	N_1	N_2	N_3	
0	39	54	69	39	54	69	39	54	68	39	54	68	39	53	53	39	53	53	39	53	53	39	52	52	38	38	38	38	38	38	38	38	38	38	38	38	38	38	38	30.0
5	39	54	68	39	53	53	39	53	53	39	52	52	38	38	38	38	38	38	38	38	38	38	37	37	37	37	37	37	37	37	35	35	35	35	35	34	34	34	34	8.9
10	39	53	53	39	52	52	38	38	38	38	38	38	38	38	38	38	38	38	36	36	36	35	35	35	34	34	34	32	32	32	29	29	29	29	29	29	-	-	-	5.2
15	39	53	53	38	38	38	38	38	38	37	37	37	36	36	36	35	35	35	33	33	33	31	31	31	29	29	29	-	-	-	-	-	-	-	-	-	-	-	-	3.7
20	38	38	38	38	38	38	37	37	37	36	36	36	33	33	33	31	31	31	28	28	28	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2.8
25	38	38	38	37	37	37	35	35	35	33	33	33	30	30	30	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2.3
30	37	37	37	36	36	36	33	33	33	28	28	28	7	7	7	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	2.0
35	36	36	36	33	33	33	27	27	27	17	17	17	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.7
40	34	34	34	30	30	30	23	23	23	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.5
45	32	32	32	26	26	26	13	13	13	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.3
51	27	27	27	18	18	18	5	5	5	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	1.2

* The permissible vertical loads are calculated by application of the calculation model according to DIN EN 12811-1, para. 10.2.3.2. To obtain the bending stiffness, internal forces and maximum load-bearing capacity,
the birdcage scaffolding with modular dimensions 2.57 x 2.57 m and different lift heights were considered:
2.00 m level height for compression forces in the standard N1 ≤ 39 kN,
1.50 m level height for compression forces in the standard 39 kN < N2 ≤ 54 kN,
1.00 m level height for compression forces in the standard 54 kN < N3 ≤ 76 kN.

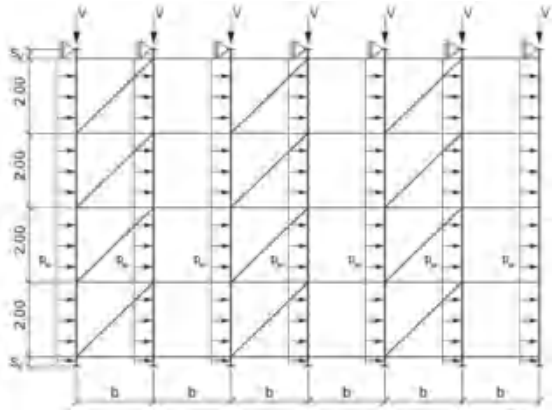
(-) With this combination of spindle extension length and horizontal load, the bending stress capacity of the spindle is exceeded.

38—the listed value or better.

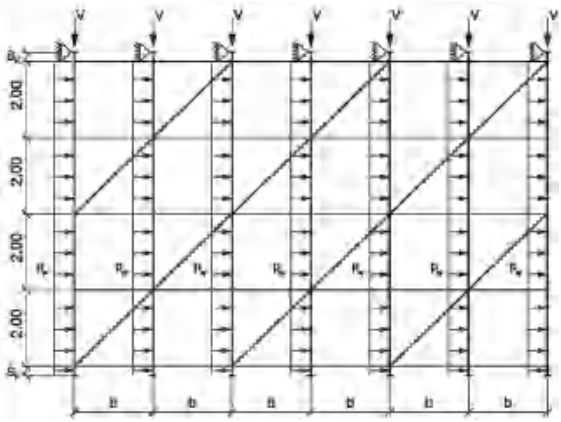
WELL MADE
SYSTEM SCAFFOLDING

SCAFFOLDING ATTACHED TO THE TOP | BASE PLATE WRBP060/ WRBPS060

Diagonal tower construction



or continuous tower construction



One diagonal brace for two bays

^{S}F Spindle extension of the base plate [cm]
^{S}K Spindle extension of the head jack [cm]
$S_K = 2/3*S_f$
Wind loads: The effective dynamic pressure, without a reduction of service life, is taken as the dynamic pressure; the area exposed to wind for all bay widths is calculated for a bay length of 3.07 m. **$p_w = q*c_f*A$**
The dead weight of the tower itself was not considered; the permissible load should be reduced by the dead weight and the ballast weight. When wind acts on the towers, a minimum load application is required to prevent sliding of the base plates. The ballast weight should be calculated.
(-) Failure of the standard or base plate due to horizontal load.

Table 3. Permissible loads in the standard

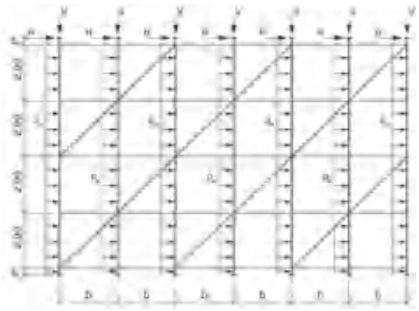
BASE PLATE 60		Permissible vertical loads in each standard in [kN] for a scaffolding attached above											
		$s_F \le 15$ cm $s_K \le 10$ cm				$s_F \le 25$ cm $s_K \le 16.70$ cm				$s_F \le 37.20$ cm $s_K \le 24.80$ cm			
Width [m]	Height [m]	Dynamic wind pressure				Dynamic wind pressure				Dynamic wind pressure			
		q = 0.0 kN/m²	q = 0.5 kN/m²	q = 0.8 kN/m²	q = 1.1 kN/m²	q = 0.0 kN/m²	q = 0.5 kN/m²	q = 0.8 kN/m²	q = 1.1 kN/m²	q = 0.0 kN/m²	q = 0.5 kN/m²	q = 0.8 kN/m²	q = 1.1 kN/m²
b = 0.73 m	3 x 2.00	26	22	20	18	23	18	15	12	19	13	10	6
	4 x 2.00	26	21	18	15	23	16	12	8	19	12	7	-
	5 x 2.00	25	19	15	10	21	14	9	-	18	10	-	-
	6 x 2.00	25	18	13	7	21	13	6	-	18	8	-	-
	7 x 2.00	25	16	10	-	21	11	-	-	18	5	-	-
	8 x 2.00	25	15	8	-	21	10	-	-	18	-	-	-
	9 x 2.00	25	12	-	-	21	7	-	-	18	-	-	-
	10 x 2.00	25	11	-	-	21	5	-	-	18	-	-	-
b = 1.09 m	3 x 2.00	26	23	21	19	23	18	16	13	19	14	11	7
	4 x 2.00	26	22	19	16	23	17	13	9	19	12	8	-
	5 x 2.00	26	21	17	13	22	15	11	6	18	10	1	-
	6 x 2.00	26	20	16	11	22	14	8	-	18	9	-	-
	7 x 2.00	25	18	13	-	22	12	-	-	18	7	-	-
	8 x 2.00	25	18	11	-	22	12	-	-	18	2	-	-
	9 x 2.00	25	15	8	-	22	9	-	-	18	-	-	-
	10 x 2.00	25	15	-	-	22	8	-	-	18	-	-	-
b = 1.57 m	3 x 2.00	26	23	22	20	23	19	16	14	19	14	11	7
	4 x 2.00	26	22	20	17	23	17	14	10	19	12	8	-
	5 x 2.00	26	21	18	14	22	16	11	7	18	11	5	-
	6 x 2.00	26	20	17	12	22	15	10	-	18	10	-	-
	7 x 2.00	25	19	14	9	22	13	6	-	18	7	-	-
	8 x 2.00	25	18	13	-	22	12	-	-	18	5	-	-
	9 x 2.00	25	16	10	-	22	10	-	-	18	-	-	-
	10 x 2.00	25	16	9	-	22	9	-	-	18	-	-	-
b = 2.07 m	3 x 2.00	26	23	22	20	23	19	16	14	19	14	11	7
	4 x 2.00	26	22	20	18	23	18	14	11	19	13	8	-
	5 x 2.00	25	21	18	15	22	16	12	7	18	11	5	-
	6 x 2.00	25	20	17	13	22	15	10	-	18	10	-	-
	7 x 2.00	25	19	15	11	22	13	7	-	18	7	-	-
	8 x 2.00	25	19	14	9	22	13	-	-	18	6	-	-
	9 x 2.00	25	17	12	-	22	11	-	-	18	-	-	-
	10 x 2.00	25	17	11	-	22	10	-	-	18	-	-	-
b = 2.57 m	3 x 2.00	26	24	22	20	23	19	17	14	19	14	11	7
	4 x 2.00	26	22	20	18	23	18	15	11	19	13	8	2
	5 x 2.00	25	21	18	15	22	16	12	7	18	11	5	-
	6 x 2.00	25	20	17	14	22	15	11	-	18	10	-	-
	7 x 2.00	25	19	15	12	22	14	7	-	18	8	-	-
	8 x 2.00	25	19	14	9	22	13	-	-	18	6	-	-
	9 x 2.00	25	18	12	-	22	12	-	-	18	2	-	-
	10 x 2.00	25	17	11	-	22	11	-	-	18	-	-	-
b = 3.07 m	3 x 2.00	26	24	22	20	23	19	17	14	19	14	11	7
	4 x 2.00	26	22	20	18	23	18	15	11	19	13	8	3
	5 x 2.00	25	21	18	15	22	16	12	8	18	11	5	-
	6 x 2.00	25	20	17	14	22	15	11	-	18	10	-	-
	7 x 2.00	25	19	15	12	22	14	8	-	18	8	-	-
	8 x 2.00	25	19	14	-	22	13	5	-	18	6	-	-
	9 x 2.00	25	18	12	-	22	12	-	-	18	2	-	-
	10 x 2.00	25	17	-	-	22	11	-	-	18	-	-	-

FREE-STANDING SCAFFOLDING | WRBP060/ WRBPS060

Table 4. Permissible loads in the standard

BASE PLATE 60			Permissible vertical loads in each standard in [kN] for a free-standing scaffolding																	
			H = 0.0 kN						H = 0.2 kN						H = 0.3 kN					
			$s_F \leq 15$ cm $s_K \leq 10$ cm			$s_F \leq 25$ cm $s_K \leq 16.70$ cm			$s_F \leq 15$ cm $s_K \leq 10$ cm			$s_F \leq 25$ cm $s_K \leq 16.70$ cm			sF ≤ 15 cm sK ≤ 10 cm			sF ≤ 25 cm sK ≤ 16.70 cm		
Width [m]	Height [m]		Dynamic wind pressure			Dynamic wind pressure			Dynamic wind pressure			Dynamic wind pressure			Dynamic wind pressure			Dynamic wind pressure		
			q = 0.0 kN/m²	q = 0.5 kN/m²	q = 0.8 kN/m²	q = 0.0 kN/m²	q = 0.5 kN/m²	q = 0.8 kN/m²	q = 0.0 kN/m²	q = 0.5 kN/m²	q = 0.8 kN/m²	q = 0.0 kN/m²	q = 0.5 kN/m²	q = 0.8 kN/m²	q = 0.0 kN/m²	q = 0.5 kN/m²	q = 0.8 kN/m²	q = 0.0 kN/m²	q = 0.5 kN/m²	q = 0.8 kN/m²
b = 1.09 m	3 x 2.00	max	27	20	15	23	16	-	26	18	13	22	13	-	24	16	-	21	9	-
		min	0	2	3	0	2	-	1	3	4	1	3	-	1	3	-	2	3	-
	4 x 2.00	max	28	17	-	25	10	-	26	15	-	23	-	-	24	13	-	21	-	-
		min	0	3	-	0	3	-	1	4	-	1	-	-	2	5	-	2	-	-
	5 x 2.00	max	26	13	-	24	-	-	24	9	-	22	-	-	22	-	-	20	-	-
		min	0	5	-	0	-	-	1	6	-	1	-	-	2	-	-	2	-	-
	6 x 2.00	max	26	-	-	24	-	-	24	-	-	22	-	-	22	-	-	19	-	-
		min	0	-	-	0	-	-	2	-	-	2	-	-	3	-	-	3	-	-
	7 x 2.00	max	25	-	-	23	-	-	23	-	-	20	-	-	20	-	-	18	-	-
		min	0	-	-	0	-	-	2	-	-	2	-	-	3	-	-	3	-	-
b = 1.57 m	3 x 2.00	max	29	23	18	25	16	7	28	21	16	23	14	-	26	19	14	22	12	-
		min	0	2	2	0	2	6	1	2	3	1	2	-	1	2	3	1	2	-
	4 x 2.00	max	29	20	13	24	13	-	28	18	-	23	10	-	26	17	-	22	-	-
		min	0	2	5	0	2	-	1	3	-	1	3	-	1	2	-	1	-	-
	5 x 2.00	max	28	16	-	24	7	-	27	14	-	23	-	-	25	13	-	21	-	-
		min	0	3	-	0	5	-	1	4	-	1	-	-	2	4	-	2	-	-
	6 x 2.00	max	28	13	-	24	-	-	27	11	-	23	-	-	25	-	-	21	-	-
		min	0	4	-	0	-	-	1	5	-	1	-	-	2	-	-	2	-	-
	7 x 2.00	max	27	-	-	24	-	-	25	-	-	22	-	-	23	-	-	20	-	-
		min	0	-	-	0	-	-	1	-	-	1	-	-	2	-	-	2	-	-
b = 2.07 m	3 x 2.00	max	29	24	19	24	17	-	28	23	18	23	15	-	26	21	16	21	13	-
		min	0	1	2	0	1	-	1	2	2	1	2	-	1	2	2	1	2	-
	4 x 2.00	max	29	22	15	24	14	-	28	20	13	23	12	-	26	19	11	21	9	-
		min	0	2	3	0	2	-	1	2	3	1	2	-	1	2	3	1	3	-
	5 x 2.00	max	29	19	-	24	9	-	28	17	-	23	-	-	26	15	-	21	-	-
		min	0	2	-	0	2	-	1	3	-	1	-	-	1	3	-	1	-	-
	6 x 2.00	max	29	16	-	24	-	-	28	14	-	23	-	-	26	13	-	21	-	-
		min	0	3	-	0	-	-	1	4	-	1	-	-	1	4	-	2	-	-
	7 x 2.00	max	29	12	-	24	-	-	27	-	-	23	-	-	25	-	-	21	-	-
		min	0	4	-	0	-	-	1	-	-	1	-	-	2	-	-	2	-	-
b = 2.57 m	3 x 2.00	max	29	25	20	24	18	10	28	24	19	23	16	-	26	22	17	21	14	-
		min	0	1	2	0	1	2	1	1	2	1	1	-	1	2	2	1	2	-
	4 x 2.00	max	29	23	16	24	15	-	28	21	14	23	13	-	26	20	13	21	10	-
		min	0	2	2	0	2	-	1	2	2	1	2	-	1	2	3	1	2	-
	5 x 2.00	max	29	20	9	24	11	-	28	19	-	23	7	-	26	17	-	21	-	-
		min	0	2	6	0	-	-	1	2	-	1	2	-	1	3	-	1	-	-
	6 x 2.00	max	29	18	-	24	-	-	28	16	-	23	-	-	26	15	-	21	-	-
		min	0	3	-	0	-	-	1	3	-	1	-	-	1	3	-	1	-	-
	7 x 2.00	max	29	15	-	24	-	-	27	13	-	23	-	-	26	-	-	21	-	-
		min	0	3	-	0	-	-	1	4	-	1	-	-	1	-	-	1	-	-
b = 3.07 m	3 x 2.00	max	29	25	21	24	18	11	28	24	19	23	16	-	26	23	18	21	15	-
		min	0	1	1	0	1	1	1	1	2	1	1	-	1	1	2	1	1	-
	4 x 2.00	max	28	23	15	23	15	-	28	22	8	23	13	-	26	21	-	21	11	-
		min	0	1	2	0	1	-	1	2	2	1	2	-	1	2	-	1	2	-
	5 x 2.00	max	28	21	-	23	12	-	28	20	-	23	8	-	26	16	-	21	-	-
		min	0	2	-	0	2	-	1	2	-	1	2	-	1	2	-	1	-	-
	6 x 2.00	max	28	18	-	23	-	-	27	12	-	22	-	-	26	-	-	21	-	-
		min	0	2	-	0	-	-	1	3	-	1	-	-	1	-	-	1	-	-
	7 x 2.00	max	28	8	-	23	-	-	27	-	-	22	-	-	26	-	-	21	-	-
		min	0	3	-	0	-	-	1	-	-	1	-	-	1	-	-	1	-	-

or continuous tower construction



One diagonal brace for two bays

S_F Spindle extension of the base plate [cm]

S_K Spindle extension of the head jack [cm]
$S_K = 2/3*S_f$

Wind loads: The effective dynamic pressure, without a reduction of service life, is taken as the dynamic pressure; the area exposed to wind for all bay widths is calculated for a bay length of 3.07 m. **$p_w = q*c_f*A$** The dead weight of the tower itself was not considered; the permissible load should be reduced by the dead weight and the ballast weight. When wind acts on the towers, a minimum load application is required to prevent sliding of the base plates.
The ballast weight should be calculated.

(-) Failure of the standard or base plate due to horizontal load.

WELL MADE
SYSTEM SCAFFOLDING

STANDARD DECKS & TYPE PLANK

Table 5. Type Plank

Load class as per EN 12811-1		O Type and U Type, 0.32 m wide, Ref. No. WRP32073 – WRP32307								O Type and U Type, 0.19 m wide, Ref. No. WRP19073 – WRP19307				Aluminium access deck, Ref. WRAA207 - WRAPL307, WRAA207		
		0.73	1.09	1.29	1.40	1.57	2.07	2.57	3.07	1.57	2.07	2.57	3.07	2.07	2.57	3.07
Perm. q. [kN/m²]	T4/T9 variant	37.6	25.3	22.5	19.7	17.5	11.4	7.5	5.0	17.7	11.4	7.5	5.0	-	-	-
	LW variant						13.3	9.3	6.5							
1 (0.75 kN/m²)		•	•	•		•	•	•	•	•	•	•	•	•	•	•
2 (1.50 kN/m²)		•	•		•	•	•	•	•	•	•	•	•	•	•	•
3 (2.00 kN/m²)		•	•		•	•	•	•	•	•	•	•	•	•	•	•
4 (3.00 kN/m²)		•	•		•	•	•	•	•	•	•	•	•	•	•	•
5 (4.50 kN/m²)		•	•		•	•	•	•	-	•	•	•	-	•	-	
6 (6.00 kN/m²)		•	•		•	•	•	-	-	•	•	-	-	-	-	
Protective scaffold and roof edge protection scaffold		•	•		•	•	•	•	•	•	•	•	•	•	•	•

PERMISSIBLE LOAD CLASSES OF THE WELL MADE SYSTEM LEDGERS

Table 6. Permissible load classes for the Well Made system ledgers

Standard type	Ledger type	Ledger length [m]	Perm. line load [kN]	Alternating decks (one-sided loading)												Unidirectional decks (two-sided loading)											
				Permissible load class with deck length [m]												Permissible load class with deck length [m]											
				1.57			2.07			2.57			3.07			1.57			2.07			2.57			3.07		
				NL	6 m²	TL	NL	6 m²	TL	NL	6 m²	TL	NL	6 m²	TL	NL	6 m²	TL	NL	6 m²	TL	NL	6 m²	TL	NL	6 m²	TL
Allround Scaffolding Ringlock	O-ledger	0.73	22.07	6	6	6	6	6	6	6	6	5+	5+	5+	4++	6	6	6	6	6	6	6	6	5+	5+	5+	4+
		1.09	10.44	6	6	6	6	6	6	6	6	5	5+	5+	4+	6	6	5	5	5	4	4	4	4	4	4	-
		1.40	6.54	6	6	6	6	6	5	5	5	4	4	4	4	4	4	4	3	3	-	3	3	-	2	3	-
		1.57	5.26	6	6	5	5	5	4	4	4	4	4	4	-	4	4	-	3	3	-	2	2	-	2	2	-
		2.07	3.12	4	4	4	3	3	-	3	3	-	2	2	-	2	2	-	1	1	-	1	1	-	1	1	-
		2.57	2.06	3	3	-	2	2	-	1	1	-	1	1	-	1	1	-	1	1	-		-	-	-	-	-
		3.07	1.46	2	2	-	1	1	-	1	1	-	-	-	-	-	-	-	B	-	-	B	-	-	-	-	-
	O-ledger, bridging	1.57	14.46	6	6	6	6	6	6	6	6	5	5+	5+	4++	6	6	6	6	6	5	5	5	5	5	5	4
		2.07	8.63	6	6	6	6	6	6	6	6	5	5	5	4+	5	5	5	4	4	4	4	4	-	3	4	-
		2.57	5.37	6	6	5	5	5	5	4	4	4	4	4	4	4	4	-	3	3	-	2	3	-	2	2	-
		3.07	3.53	4	4	4	4	4	-	3	3	-	3	3	-	3	3	-	1	2	-	1	1	-	1	1	-
	U-ledger	0.73	19.01	6	6	6	6	6	6	6	6	5+	5+	5+	4++	6	6	6	6	6	6	6	6	5+	5+	5+	4++
		1.09	14.09	6	6	6	6	6	6	6	6	5+	5+	5+	4++	6	6	6	6	6	5	5	5	5	4	4	4
		1.29	8.76	6	6	6	6	6	6	6	6	5	5	5	4+	5	5	5	4	4	4	4	4	-	3	3	-
		1.40	7.03	6	6	6	6	6	5	5	5	5	4	4	4	4	4	4	4	4	-	3	3	-	3	3	-
	U-ledger, reinforced	1.40	17.34	6	6	6	6	6	6	6	6	5+	5+	5+	4+	6	6	6	6	6	6	6	6	5	5	5+	4+
		1.57	10.42	6	6	6	6	6	6	6	6	5+	5+	5+	4+	6	6	5	5	5	4	4	4	4	4	4	-
		2.07	13.00	6	6	6	6	6	6	6	6	5+	5+	5+	4++	6	6	6	6	6	5	5	5	4	4	5	4
		2.57	8.40	6	6	6	6	6	6	6	6	5	5	5	4+	5	5	5	4	4	4	4	4	-	3	4	-
		3.07	5.00	6	6	5	5	5	4	4	4	4	4	4	-	3	4	-	3	3	-	2	3	-	1	2	-
	U-ledger, bridging	1.57	15.16	6	6	6	6	6	6	6	6	5+	5+	5+	4++	6	6	6	6	6	5	5	5	5	5	5	4
		2.07	8.65	6	6	6	6	6	6	6	6	5	5	5	4+	5	5	5	4	4	4	4	4	-	3	4	-
		2.57	5.12	6	6	5	5	5	4	4	4	4	4	4	-	4	4	-	3	3	-	2	3	-	1	2	-
		3.07	3.59	4	4	4	4	4	-	3	3	-	3	3	-	3	3	-	2	2	-	1	1	-	1	1	-

NL—Nominal load as per EN 12811-1 Table 3, as evenly distributed area load.
TL—Partial area load as per EN 128111 Table 3, for load classes 4, 5 and 6.
6 m²—Nominal load limited to 6 m² in a birdcage scaffolding as per EN 12811-1 Section 6.2.2.6:.
"The load on the supporting components of a birdcage scaffold shall be calculated by assuming that the permissible load acts on an area of 6 m² in combination with a load of 0.75 kN/m² over the remaining area."
+ The load-bearing capacity of the deck has been achieved, the ledger is classified one load class higher.
++ The load-bearing capacity of the deck has been achieved, the ledger is classified two load classes higher.

MAJOR
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ATTENTIVELY

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